Comparison of expert, deterministic and Machine Learning approach for landslide susceptibility assessment in Ljubovija Municipality, Serbia

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Landslide Susceptibility Assessment is becoming a very productive research area, wherein different modeling approaches are practiced to delineate zones of the high-low likelihood of landslide occurrence. However, there is no strong consensus on which approach is the most adequate. The reason behind the lack of the general view on the performance of different approaches could be partially explained by the particularity of each study. To evaluate the efficiency of different approaches they need to be applied under the same conditions for the same study area. Herein, we examined three different approaches, including expert, deterministic and Machine Learning, on the example of Ljubovija Municipality in western Serbia. The study area has been known as susceptible to landslides, and represents good ground for assessing the chosen methods. It is represented by complex geology, prone to landslides that are commonly hosted in thick weathering crust of Paleozoic formations, composed of schists and meta-sediments. Under extreme triggering conditions, such as the one that unfolded in May 2014, these thick weathering crusts saturate, and give way to a variety of landslide and flash-flood processes that we will be focusing on in this study. The application of the expert-approach, through Analytical Hierarchy Process provided a rough assessment map. The deterministic model, which couples simple infinite slope and hydrological model, provided us with lower quality results, when compared to the expert-based one. This could be explained by the assumptions used in the model are too simplistic to generically model a wide range of landslide typology. Finally, Machine Learning approach, using the Random Forest algorithm, provided significantly better results and showed that it can cope with versatile landslide typology over larger scales. Its AUC performance is about 0.75 which is considerably outperforming the AUC values of the other two models, which were up to 0.55, i.e. at the level of random guess.

Keywords: landslide susceptibility, Analytical Hierarchy Process, deterministic, Machine Learning, Random Forest
1. Introduction

There are various approaches to Landslide Susceptibility Assessment (LSA), including expert-driven, deterministic, statistic and Machine Learning methods (Guzzetti et al., 1999). Which of them will perform better in a particular case is difficult to determine beforehand, so trial-and-error and comparative studies are always necessary. Reasons that underpin the complexity of this topic are many: quality of input data, suitability of the model, but one of the main reasons lies in the nature of landslide phenomena itself. For instance, the massive rainfall event can simultaneously trigger various types of landslides, which can be different in their mechanisms, size, character, depth, etc. It is, therefore, difficult for any LSA approach to cope with all these circumstances. Herein, one such case was addressed, linked to a heavy rainstorm and subsequent flooding and massive landsliding that occurred in Serbia in May 2014. In particular, it addresses Ljubovija Municipality in western Serbia, as one of the areas that suffered the heaviest aftermath, in respect to the number of landslides and total damage, but also in respect to a very diverse landslide typology: shallow and deep-seated, rapid and slow, reactivated and newly-formed. It was therefore convenient to use the area of Ljubovija for testing various LSA approaches and comparing their performance, fulfilling the principal objective of this study. Three different approaches were applied and compared:

- for expert-driven or heuristic approach – Analytical Hierarchy Process (AHP) was used
- for deterministic or physical approach – Stability Index (SI) modelling was implemented
- for Machine Learning data-driven approach – Random Forest (RF) algorithm was implemented.

Heuristic methods are mainly qualitative and subjective, as they depend on the judgment of decision makers (experts) in particular research area. The most common heuristic approach is based on analyzing landslide inventory to identify sites of similar geological and geomorphological properties. Many authors do not recommend the use of heuristic methods for hazard, but only for susceptibility zoning (Fell et al., 2008; Cascini, 2008; Barredo et al., 2000; Ercanoglu et al., 2008). The reason for this is that the results are partly subjective, and depend on the experience of experts, which implies insufficient knowledge and sometimes generalization of the results in a particular area (Atkinson and Massari, 1998; Ayalew and Yamagishi, 2005; Yalcin, 2008; Yilmaz, 2009). On the other hand, it is generally accepted that heuristics could and should be used for preliminary levels of research-regional studies (Soeters and van Westen, 1996; Fell et al., 2008; Guzzetti et al., 1999). Some methods include ranking and weighting, which implies semi-quantitative approach (Ayalew and Yamagishi, 2005). Analytic Hierarchy Process (AHP), first time used by Saaty 1980, is one of the semi-quantitative methods that allows subjective as well as objective factors to be
considered in the decision-making process. Successful examples of AHP implementation were reported by numerous researchers, but the main impression remains that it is more suitable for preliminary assessment on regional scales (Komac et al., 2006).

Deterministic modeling on regional scales is difficult due to a number of reasons: amount and sampling density of required physical parameters, which can be very costly, as well as the appropriateness of applied geometrical and physical modeling assumptions to the given case study. The number of physical and geometric parameters is therefore reduced, at the expense of simplifying the model. Ultimately, only the simplest Limit Equilibrium (LE) slope stability methods comply, e.g. infinite slope model, where geometry and landsliding physics are trivial. The LE models are usually coupled with simplified hydrological models. The impression is that deterministic methodology in LSA is rather limited and rounded. The only differences that are practiced come down to the variation of coupled hydrological models or small adjustments or extensions to the original LE assumptions of infinite slope model in regional assessment (Montrasio and Valentino 2016). The pioneering work in deterministic LSA was performed by Montgomery and Dietrich, 1994, where the general LE principle is implemented through the Stability Index (SI), coupled with a transient steady-state groundwater flow model. There were further attempts to sophisticate the LE conditions by introducing additional forces, or dimensionless factors (Pack et al., 2001), involve stochastics through input data distribution estimations (Pack et al., 2001), but mostly by including more complex coupled hydrological model: 3D-Richards equation; temporal, frost, and unsaturated terrain effects (Rigon et al., 2006; Endrizzi et al., 2013; Oh and Lu, 2015). Most of these researchers further emphasized that deterministic approach is more apt for local modeling, i.e. site-specific scales than regional, whereas regional scales face various limitations. Considerable generalization of landsliding mechanism is therein always needed, entailing that only shallow slides, triggered solely by rainfall discharge, comply with regional assessment. Partly, this was the case with landslides triggered in Ljubovija in May 2014, except that there were many flows in combination with shallow slides, so the applicability of discriminant modeling is relatively sound for the case of Ljubovija.

The capability of the ML techniques to learn and derive patterns of the phenomena of interest by exploring unknown relations between considering casual/conditional factor and the variable of interest has made these techniques very popular in many various fields of science, including the LSA. Many research studies have already successfully applied various ML techniques for LSA, such as Decision Tree (DT), Neural Network (NN), Support Vector Machine (SVM) and Logistic Regression (LR) (Lifeng and Youshui, 2006; Marjanović et al., 2011; Pradhan, 2013) and more recent RF. As a relatively new ML technique, RF, which generates a model by combining the results of a set of simpler DT-based models, received increasing attention in the LSA community (Catani et al., 2013;
Goetz et al., 2015; Trigila et al., 2015; Vorpahl et al., 2012). One of the first published researches, that addresses the application of RF for the LSA, was performed by Vorpahl et al. (2012). In order to model LSA, they used five historical landslide inventories and seven terrain attributes derived from a Digital Elevation Model (DEM). They compared eight different techniques and concluded that RF and Boosted Regression Tree (BRT) had better performance compared to Generalized Linear Models (GLM), General Additive Models (GAM), Multivariate Adaptive Regression Splines (MARS), NN, DT (CART) and Maximum entropy method (MEM). Goetz et al. (2015) compared the performance of six various statistical and ML techniques for LSA: GLM, GAM, weights of evidence (WOE), SVM, RF, and Bootstrap aggregated classification trees with penalized discriminant analysis (BPLDA). They conducted the study using three study areas and 11 terrain attributes derived from DEM, as predictor variables. The derived results also suggested that the RF was one of the techniques with best prediction performance. Trigila et al. (2015) created LR and RF-based LSA models, using highly urbanized study area and in addition to terrain characteristic derived from DEM they used Distance to stream, Lithology, Land use/land cover, Agricultural terraces, and Wildfires as predictors. They found that both techniques equally provide excellent results. Considering some of the advantages of RF compared to other ML techniques (such as that RF does not overfit due to the Law of Large Numbers and its capability to handle scarce attributes (Breiman, 2001)) and that in most of the literature related to the LSA, RF technique was identified as one of the best performings, this technique was selected to represent Machine Learning data-driven approach for this research the case study.

2. Methods

Herein, all three modeling methods, i.e. expert-based, deterministic, and Machine Learning, will be elaborated, as well as the principle of their evaluation against the existing landslide inventory, and related evaluation metrics.

2.1. Expert method – Analytical Hierarchy Process (AHP)

Analytic Hierarchy Process (AHP) is one of the most popular Multi-Criteria Decision-Making (MCDM) tools for formulating and analyzing decisions (Saaty, 1980; Ishizaka and Labib, 2011). This methodology calibrates the numeric scale for the measurement of quantitative as well as the qualitative influence of separate attributes on the phenomena at hand. The scale of influence ranges from 1/9 for “least valued than 9”, to 1 for “equal”, and to 9 for “absolutely more important than” covering the entire spectrum of the comparison (Vaidya and Kumar, 2006). Multiple criteria decision problem is first broken down into its component parts of which every possible attribute is arranged into multiple
hierarchical levels. The AHP consists of three main operations, including hierarchy construction, priority analysis, and consistency verification (Ho, 2008). This approach is widely used in the assessment of landslide susceptibility (Komac, 2006; Marjanovic et al., 2011; Yalcin, 2008; Pourghasemi et al., 2012). The implementation of AHP methodology in the assessment of landslide susceptibility firstly requires finding of interdependencies between most important influential attributes, in this case: engineering geological units, slope, elevation, distance from hydrogeological borders, stream distance, land cover, aspect, and erodibility. It is highly recommended to normalize the values of input attributes and classify them into a specific number of classes (e.g. herein 1–5 class range was used, meaning that 1 is the least likely to host landslides and 5 is highly likely to host landslides). AHP principle further implies generation of a comparative matrix of selected attributes with the 1–9 scale of influence. It is inversely symmetric ($a_{ij} = a_{ji} = 1$) in respect to the main diagonal. Reclassified and ranged attributes ($A_i$), with their weights ($W_{Ai}$), give final impact on susceptibility model (LSM). The sum of the weights of all the parameters is equal to 1, or 100% (Saaty, 1980; 1987; Saaty and Vargas 2001). Consistency of attribute comparison is defined by Consistency Ratio – CR (Bhushan and Rai, 2004; Subramanian and Ramanathan, 2012; Saaty, 1980). This final operation controls the comparison judgment. Comparisons made by AHP method are subjective, and some inconsistency is tolerated. Consistency Index, $CI$, is calculated as:

$$CI = (\lambda_{\text{max}} - n) / (n - 1)$$  

where $\lambda_{\text{max}}$ is the maximum weight of the judgment matrix and $n$ is the number of attributes. Accordingly, $(\lambda_{\text{max}} - n)$ can be considered as a measure of the degree of inconsistency, which is normalized with $n - 1$ (Malczewski et al., 1997). The next step is to calculate Consistency Ratio, $CR$:

$$CR = CI / RI$$  

where $RI$ is the consistency index of a randomly generated pairwise comparison matrix, that depends on the number of criteria being compared. Saaty and Vargas (2001) suggest the value of $CR$ should be less than 0.10. If the values are above 0.10 revision of the judgment in the matrix is required.

2.2. Deterministic method – Stability Index (SI)

Stability Index (SI) concept consists of a typical slope stability model, coupled with a simple hydrological triggering model. Slope stability model is reduced to infinite slope geometry with simple Limit Equilibrium conditions, wherein slip surface is parallel to the topographic slope $\theta$, the model is irrespective to the soil thickness, cohesion and pore pressure are canceled by soil weight to become unitless variables, effective stress Coulomb-Mohr’s failure criteria is in power. General slope stability principle implies the static balance of driving and resisting forces that act on a slip surface. Their ratio is represented by the Factor of
Safety ($FS$), which is further coupled with the triggering model by introducing boundary conditions of soil transmissivity vs. rainfall discharge $T/R$, wherein the stationary flow under steady-state conditions is assumed. Subsequently, stochastics is introduced into $FS$ by ranging unit weight $\gamma_{soil}$, cohesion $c_{soil}$, friction angle $\varphi_{soil}$ and transmissivity $T$, using uniform min/max distribution, to simulate the worst and best-case extremes (Eq. (3)). When $T/R$ flux is maximal and strength parameters ($c_{soil}$ and $\varphi_{soil}$) are at their lowest (assuming that water to soil weight ratio $r$ is constant over designated rock unit) the worst-case extreme implies that $SI$ will equal $FS_{min}$, and all areas with $FS_{min} > 1$ will be unconditionally stable, whereas values bellow 1 imply failure with appropriate level of probability. On the other hand, best-case extreme considers minimal $T/R$ flux and highest strength parameters, wherein all areas with $FS_{max} < 1$ will be definitively unstable ($SI=0$), whereas those with values greater than 1 will also have some low probability to fail. All intermediate $FS$ values with some probability of failure ($FS_{min} \leq 1$ and $FS_{max} \geq 1$) are further depending on slope angle $\theta$ and discharge area $a$, variables that are available for calculation for wider, regional scale areas in GIS. In general, the lower the $SI$ is, the lesser the stability and vice-versa. Herein, the modeling was performed in MapWindow via extension SIN-MAP – Stability INdex MAPping (Pack et al., 2001).

$$FS = c_{soil} + \cos \theta \left(1 - \min \left(\frac{Ra}{T_{sin}^3}, 1\right) r \right) \frac{\sin \varphi_{soil}}{\sin \theta}$$

(3)

2.3. Machine Learning method - Random Forest (RF)

RF is a relatively new ML technique, proposed by Breiman (2001), which has already achieved wide popularity in the Geoscience community with solving various classification and regression problems. In this study, the spatial phenomenon of landslides is considered as a classification problem, with binary target variable $l_i$ (1 = the location $i$ is occupied with a landslide or 0 = not occupied with landslide). The main goal of the RF technique is to find the classification function, $c_i (a_1, a_2, ..., a_j) \rightarrow l_i$, which maps landslide and non-landslide class at a particular location. Consequently, each location defined by $30 \times 30$ m grid cell is presented as an instance $c_i$, which contains corresponding values of all conditioning factors $a_j$ (factor such as: slope, erodibility, lithology, distance from hydrogeological borders, stream distance, aspect, land use elevation, ...) and a landslide $l_i$ as a target variable to be classified.

After defining the proper dataset which represents the entire study area as a set of instances $\{(c_i, l_i)\}, i = 1, ..., 387207$, the next step was to create two sub-datasets, for training and testing. The subset of the dataset used for finding the classification function, i.e. the relationship between the attributes and a landslide class is called the training dataset. The second subset, testing dataset, is inde-
dependent of the *training dataset*. It was used to validate the obtained RF-based model by comparing the model classification outcome and real information of landslide. In order to provide a more informative *training dataset*, considering the relatively small number of instances which represent the landslide areas (1 475 instances, 0.38% of entire study area), the following sampling procedure proposed by Marjanović et al. (in press) was performed. A half of all instances which represent landslides were randomly selected as training landslide samples, ensuring that they are distributed across all landslide polygons from the inventory, with considering the size of each landslide polygon. The same number of training samples was selected as non-landslides, ensuring that they are distributed across all previously defined polygons (polygons were delineated on the basis of landslide expert judgment on which locations are highly unlikely to host landslides, such as ridge-lines, compact rocks, and flat surfaces).

Using this sampling strategy, both classes (landslide - 1 and non-landslide - 0) are equally represented, avoiding the possibility that the RF model can be affected by the majority classes, which in this study were the locations without landslides. The rest of the instances that have not been sampled for training dataset were used in the testing dataset. *Training dataset* contained 1 474 instances, 737 instances with landslides and 737 instances without landslides, whereas *testing dataset* contained 385 733 instances, 738 instances with landslides and 384,995 non-landslide instances.

The next step was to build the RF-based model. Generally, using *training dataset* RF generates a model by combining the results of a set of simpler decision trees (DT) based models (Fig. 1). Each DT in RF (Fig. 1) is constructed over a new subset of *training dataset* that contains randomly selected $p$ ($p < 1 474$) instances of the initial *training dataset*. Furthermore, each DT in RF is grown

![Figure 1. Schematics of the Random Forest implementation.](image-url)
using methodology proposed by Breiman (1984), i.e. by selecting attribute among \( q \) \((q < M, M = \text{number of all considering attributes})\) randomly selected attributes, not among all considering attributes, as is usually done in DT.

Finally, a RF obtains a classification outcome from each DT, and then further classifies using majority class for each instance or using the defined threshold of class 1 (landslide) probability.

The efficient application of most ML techniques requires selection of an optimal combination of function parameters, which are in the case of RF two parameters, the number of DT in the forest and the number of randomly selected factors that will be considered per each node in the DT. These parameters were found in the process of cross-validation in which a model is trained on a portion of the training dataset and validated on the remaining part, so that fitting parameters are optimized only over training instances.

Validation was performed on the testing dataset by comparing the models’ outcomes with real information on landslides using the several validation measures: true positive rate (TPrate), false positive rate (FPrate), true negative rate (TNrate), false negative rate (FNrate) and Area Under Receiver Operating Characteristic Curve (AUC) (Bradley, 1997).

3. Case study area and data

The territory of the Municipality of Ljubovija is located in the western part of Serbia. It covers an area of 331.47 km\(^2\) with a population of 3929. The wider

![Figure 2. Geographical position and relief map (DEM 30 m resolution) of Ljubovija Municipality.](image)
area can be characterized with moderate-continental to continental climate (Fig. 2). The Drina River and its valley dominate the landscape, together with its tributaries. The right side of the valley is quite steep, and exposed to slope processes: weathering, deluvial processes, gulling, landslides, and flash-floods. One of the longest tributaries is The Ljubovića River, which is known for its torrential character. In terms of the relief, the territory is predominantly hilly and mountainous. The highest peak is Tornička Bobija (elevation of 1268 m), while the lowest part is the valley of The Drina River in the coastal area (158 m). The relative elevation difference between is 1110 m, with an intermediate slope angle around 18°, but it is apparent that the terrain is mostly steep, with a maximum slope angle of 65°.

Regarding the geological settings, most of the terrain consists of Paleozoic formations (quartz sandstones and shales of lower crystallinity – phyllites and slates), Triassic formations (massive and plate crystalline limestones, dolomites, dolomite limestone), Jurassic formations (conglomerates, claystones and serpentinites) and Quaternary deposits (deluvial-proluvial and alluvial sediments). Depending on the thickness of the weathering crust, different types of landslides could be expected to occur. It is typical that landslides hosted in the weathering crust mobilize after saturation, preceded by continuous rainfall input or a heavy rainfall, such as the one that took place in May 2014. During this event, maximum 72 h precipitation in the entire Serbia was recorded in this region, exceeding 200 mm. For comparison, the average amount of annual rainfall for the reference period (1981–2010) was 786.96 mm, while in 2014, in extreme conditions, it exceeded 1249.98 mm. The average sum by a month in May for the reference period is 70.7 mm, and in May 2014 that sum was 234.5 mm, respectively 214.95% more than average. These conditions led to triggering of many new, and reactivating some of the suspended landslides. There were 271 landslides registered by remote sensing, and 70 landslides mapped in the field (Marjanović et al., 2016). Approximately, a half of all landslides are active landslides, and only 2% are stabilized or dormant (http://geoliss.mre.gov.rs/beware/).

4. Input data

Landslide inventory map that shows the location of discernible landslides is essential in landslide susceptibility mapping and hazards modeling (Dahal and Hasegawa, 2008; Pourghasemi et al., 2012). The total of 271 landslide occurrences inventoried across the entire study area, with the approximate size ranging from 15 m² to 143,780 m². As for the landslides recorded in the field, 49% were identified as active, 46% as suspended, and 5% of landslides as inactive (stabilized, temporally dormant). During the initial rasterization of the inventory at 30 m resolution, these 271 occurrences were represented by only 213 30 m × 30 m landslide pixels, which was insufficient for modeling and evaluation.
Thus, a 5-m buffer zone around each landslide was also considered as a landslide area, which increased the total number of landslide pixels to 265.

In order to assess susceptibility, as a spatial likelihood of occurrence of landslide events, it is necessary to determine landslide conditioning factors. The preparatory variables which make the slope susceptible to failure without initiating it, and thereby tending to put the slope in a marginally stable state, are usually considered constant, regarding the life of the landslide process, which is sometimes measured in decades. Three groups of attributes were considered, i.e. morphologic, geological and environmental.

4.1. Morphologic attributes

A Digital Elevation Model (DEM) is representing the terrain and it has been used to derive various morphometric parameters which influence slope instability. A DEM of the study area with 30 m × 30 m cell size was generated using data from the Republic Geodetic Authority of Serbia. Adopted resolution was the optimal considering the fact that for regional LSA, 30 m cell resolution is enough detailed for DEM and the grid size is proper and compatible with other data sources, for example, Engineering Geological map at 1:300 000 scale. Morphometric thematic data layers such as slope, aspect, and elevation were compiled based on DEM.

Slope ($A_2$) – The main parameter of the slope stability analysis is the slope degree (Lee and Min 2001), and it is very commonly used in the landslide susceptibility studies (Lee, 2005; Yalcin, 2008; Nefeslioglu et al., 2008). Greater angles propose higher instability of the slopes and vice-versa, but with restriction to a particular rock type (e.g. solid rocks are expected to be stable even in steep slopes, while slopes in clay do not need a steep angle to host instability). The most of the landslides were located in range with an inclination of 10–20° (Fig. 3a).

Slope aspect ($A_7$) – This attribute (Fig. 3e) represents spatial exposure of the ground element to diurnal solar path and it ranges from 0° to 360°. Slope instabilities are affected by slope aspect because it is associated with the insolation which influences physical-mechanical decomposition of rocks, and moisture in the soil, which is important for landsliding (Suzen and Doyuran, 2004; Komac, 2006). It is assumed that northern and eastern slopes are the most inconvenient for stability (highest water retention and the thickest weathering crust), while the southern and western are the most favorable, due to different degrees of insolation.

Elevation ($A_3$) – Elevation is another feature physically related to landslides since potential energy is increasing with altitude, and thereby contributes landsliding. It is represented by DEM (Fig. 2).

Distance from the stream ($A_5$) – This attribute was generated from a digitized hydrography (Fig. 3c). Distance to stream has an influence on slope stability, due to the fact that deformation and failure processes develop regressively upslope
under the vertical and lateral influence of the linear erosion of streams. Landslide occurrences along streams are often because streams are eroding and undercutting foot of the slope, and saturating its lower parts (Dai et al., 2001; Saha et al., 2002). In this respect, it is assumed that areas closer to the stream lines are more prone to instabilities than remote ones, thus landslide susceptibility decreases with distance from the local watercourse.

4.2. Geological attributes

Engineering geological units \((A_1)\) – Different lithological units have essentially different physical-mechanical behavior and therefore different impact on slope stability. These were aggregated based on the engineering geological criteria into units of similar genesis and physical-mechanical properties (Fig. 4). This data was acquired from the engineering geological map of the study area at 1:300 000 scale. The landslides occurrence is likely in crystalline metamorphic rocks, which typically have a thick weathering crust, as well as softer lacustrine sediments. Flysch and ultramafic formations also hosted some occurrences, but to a much lesser degree.

Distance from hydrogeological borders \((A_4)\) – This is a parameter that was introduced to emphasize the influence of hydrogeological boundaries on the landslide process and the rate of change of hydrogeological function between adjacent rock masses (Fig. 3b). Zones closer to the boundary are more susceptible to landslide occurrence, which was confirmed by the field research.

Erodibility \((A_8)\) – attribute was calculated from \(A_1\) and \(A_6\) according to Gavrilović (1971) methodology (Fig. 3f). Greater potential of erodibility is tightly related to landsliding process. Directly, landslides themselves are caused by erosional processes, and indirectly, landslides tend to develop along linear features such as gullies or ravines.

4.3. Environmental attributes

Land use \((A_6)\) – This thematic attribute was obtained from CORINE Land cover 2012 data (Fig. 3d). Vegetated areas decrease the susceptibility to landsliding due to the increased root cohesion. Water retention is also greater in vegetated than bare surfaces, which is another stabilizing aspect. The land cover does not play a vital role during rapid and intense rainfall events, but they can be of importance for the long-term accumulation of water in soil (Yalcin et al., 2011).

Deterministic modeling approach involves more localized data. First, the samples are taken in-situ, and then used for a set of laboratory tests to obtain standard geotechnical parameters (bulk density, friction angle, cohesion, etc.) per each geological unit. Subsequently, these are generalized (extrapolated within the 30 m gridded rasters) over wider areas of presumably similar characteristics, called engineering geological units. The most important parameters to
Figure 3. Input attributes: (a) $A_1$, Slope, (b) $A_2$, Distance from hydrogeological borders, (c) $A_3$, Stream distance, (d) $A_4$, Land use (112/131 – artificial surfaces; 211/243 – agricultural areas; 311/313 – forests; 321/324 – scrub and herbaceous vegetation association; 333 – sparsely vegetated areas; 511 – water courses) (e) $A_5$, Aspect, (f) $A_6$, Erodibility.
characterize an EG unit included unit weight, cohesion, friction angle, and hydraulic conductivity. Sampling and testing over wider areas would be very costly, which is why these parameters had to be estimated, based on previous research and generally suggested values (Fell et al., 2008). The EG units and their related parameters are outlined in Fig. 4, while Tab. 1 shows their values. The EG units are interpreted on the basis of Engineering Geological map. Rock units of similar mechanical properties were aggregated into appropriate complexes. Each complex unit was assigned with an appropriate range of min/max expected values of according strength and triggering parameters. Hydraulic conductivity was used to estimate the $T/R$ ratio. For this purpose, min/max expected values of according strength and triggering parameters. Hydraulic conductivity was used to estimate the $T/R$ ratio. For this purpose, official monthly extreme precipitation (www.hidmet.gov.rs) for Ljubovija was used for estimating critical recharge $R$. Given that maximal and minimal monthly rainfall equals 394.3 mm and 16.4 mm, respectively, and assuming that $T$ equals hydraulic conductivity, $T$ was recalculated on monthly basis, i.e. mm/month, instead of the typical cm/s (see Tab. 1 – Hydraulic conductivity). $T$ was then divided by $R$ giving the respective minimal and maximal $T/R$. The following EG units, with according adopted parameters, were finally defined (see Tab. 1 – Description).

Figure 4. Engineering geological (EG) units of similar mechanical properties TMP.
Table 1. Engineering geological units of similar mechanical properties.

<table>
<thead>
<tr>
<th>EG units</th>
<th>Rock unit</th>
<th>Density [g/cm³]</th>
<th>Cohesion/ unit weight [unitless]</th>
<th>Friction angle [°]</th>
<th>Hydraulic conductivity [cm/s]</th>
<th>T/R [unitless]</th>
<th>Description</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>Flysch</td>
<td>1900</td>
<td>0.5–5.5</td>
<td>24–34</td>
<td>0.000001</td>
<td>0.065–1.58</td>
<td>alternating claystone-sandstone sequences, impermeable to superficial recharge, medium-low strength</td>
</tr>
<tr>
<td>2</td>
<td>al-pr</td>
<td>1800</td>
<td>0–0.05</td>
<td>21–31</td>
<td>0.01</td>
<td>657.37–15805</td>
<td>gravel-sand-clay soil deposits, very permeable, low strength</td>
</tr>
<tr>
<td>3</td>
<td>Limestone</td>
<td>2300</td>
<td>0.5–3</td>
<td>30–58</td>
<td>0.001</td>
<td>65.739–15805</td>
<td>solid rock, permeable, high strength</td>
</tr>
<tr>
<td>4</td>
<td>Ophiolite</td>
<td>1800</td>
<td>0.6–3</td>
<td>26–46</td>
<td>0.00001</td>
<td>0.657–15.8</td>
<td>solid rock mixture (chert, sandstone, limestone, diabase), permeable, moderate strength</td>
</tr>
<tr>
<td>5</td>
<td>Andenzite</td>
<td>2100</td>
<td>0.3–2</td>
<td>25–47</td>
<td>0.00001</td>
<td>0.657–115.8</td>
<td>solid rock, permeable, moderate strength</td>
</tr>
<tr>
<td>6</td>
<td>Granit</td>
<td>2400</td>
<td>0.3–7</td>
<td>25–65</td>
<td>0.00001</td>
<td>0.066–15.8</td>
<td>solid rock, impermeable, high strength</td>
</tr>
<tr>
<td>7</td>
<td>Sandstone</td>
<td>1900</td>
<td>0.4–3</td>
<td>20–40</td>
<td>0.00001</td>
<td>0.657–115.8</td>
<td>solid-soft rock, permeable, moderate to low strength</td>
</tr>
<tr>
<td>8</td>
<td>Schist</td>
<td>2100</td>
<td>0.4–1.5</td>
<td>20–40</td>
<td>0.000001</td>
<td>0.065–15.8</td>
<td>solid rock, but mantled with clayey crust, relatively permeable, moderate to low strength</td>
</tr>
<tr>
<td>9</td>
<td>Piroclasitite</td>
<td>1600</td>
<td>0.8–3</td>
<td>45–65</td>
<td>0.000001</td>
<td>0.065–1.58</td>
<td>soft rock, porous but impermeable, moderate-low strength</td>
</tr>
<tr>
<td>10</td>
<td>Diabaze</td>
<td>2300</td>
<td>0.6–5</td>
<td>28–50</td>
<td>0.000001</td>
<td>0.065–1.58</td>
<td>solid rock, impermeable, high strength</td>
</tr>
<tr>
<td>11</td>
<td>Serpentinite</td>
<td>2700</td>
<td>0.4–8</td>
<td>60–70</td>
<td>0.000001</td>
<td>0.065–1.58</td>
<td>solid rock, impermeable, high strength</td>
</tr>
<tr>
<td>12</td>
<td>Marl</td>
<td>2000</td>
<td>0.6–2</td>
<td>35–55</td>
<td>0.00001</td>
<td>0.065–1.58</td>
<td>soft rock, impermeable, low strength</td>
</tr>
</tbody>
</table>
5. Results

Applying AHP approach first required ranging and scoring of input attributes. All numeric input attributes were separated into five classes with scores 1 to 5. Range intervals for these classes were adjusted according to the landside density within each class. Score 1 suggests that particular class does not contribute to landsliding, while score 5 does. Intermediate scores 2, 3 and 4 are evenly separated intervals between these two extreme classes. Nominal attributes, such as \( A_7 \) and \( A_1 \), did not underwent reclassification by ranging, but some subjective scoring had to be provided. The most difficult was the scoring of the engineering geological units, but two extremes were easily identified: 1 alluvial deposits and solid intact rock formations, and 5 deluvial deposits and Paleozoic low-crystalline schists. All intermediate classes were scored using the subjective expert judgment. Given these preprocessed inputs, comparison AHP matrix was constructed (Tab. 2). EG units \( (A_i) \) were defined as the most important attribute, followed by slope angle \( A_2 \), and so forth to erodibility \( A_8 \), which was given the smallest influence on the final model.

After defining relation within different input \( A_i \) attributes, matrix normalization was performed (Tab. 3). Each value was divided by the total sum along the column. The result across rows is the eigenvector, which reflects the impact or weight on landslide susceptibility of each attribute \((W_{A_i})\). Consistency Ratio value \( CR = 0.045 \) indicated a good and logical pair-wising and scoring of the attributes.

Superposition of each weighted attribute gives the final additive Landslide Susceptibility Model (LSM) (Fig. 6a). The evaluation was made in comparison with the landslide database (Tab. 4, Fig. 5).

\[
LSM = W_{A_1} \times A_1 + W_{A_2} \times A_2 + W_{A_3} \times A_3 + W_{A_4} \times A_4 + W_{A_5} \times A_5 + W_{A_6} \times A_6 + W_{A_7} \times A_7 + W_{A_8} \times A_8
\]

\[
= 0.26 \times A_1 + 0.21 \times A_2 + 0.20 \times A_3 + 0.14 \times A_4 + 0.08 \times A_5 + 0.06 \times A_6 + 0.03 \times A_7 + 0.02 \times A_8 \quad (4)
\]

The \( SI \) model was prepared based on the spatial aggregation and ranging of strength and triggering parameters. Ranging was performed by readjusting ref-

---

Table 2. AHP comparison matrix.

<table>
<thead>
<tr>
<th></th>
<th>( A_1 )</th>
<th>( A_2 )</th>
<th>( A_3 )</th>
<th>( A_4 )</th>
<th>( A_5 )</th>
<th>( A_6 )</th>
<th>( A_7 )</th>
<th>( A_8 )</th>
</tr>
</thead>
<tbody>
<tr>
<td>( A_1 )</td>
<td>1.00</td>
<td>1.00</td>
<td>2.00</td>
<td>2.00</td>
<td>4.00</td>
<td>6.00</td>
<td>8.00</td>
<td>9.00</td>
</tr>
<tr>
<td>( A_2 )</td>
<td>1.00</td>
<td>1.00</td>
<td>1.00</td>
<td>2.00</td>
<td>3.00</td>
<td>4.00</td>
<td>7.00</td>
<td>8.00</td>
</tr>
<tr>
<td>( A_3 )</td>
<td>0.50</td>
<td>1.00</td>
<td>1.00</td>
<td>2.00</td>
<td>3.00</td>
<td>5.00</td>
<td>6.00</td>
<td>7.00</td>
</tr>
<tr>
<td>( A_4 )</td>
<td>0.50</td>
<td>0.50</td>
<td>0.50</td>
<td>1.00</td>
<td>2.00</td>
<td>4.00</td>
<td>5.00</td>
<td>7.00</td>
</tr>
<tr>
<td>( A_5 )</td>
<td>0.25</td>
<td>0.33</td>
<td>0.33</td>
<td>0.50</td>
<td>1.00</td>
<td>2.00</td>
<td>4.00</td>
<td>6.00</td>
</tr>
<tr>
<td>( A_6 )</td>
<td>0.17</td>
<td>0.25</td>
<td>0.20</td>
<td>0.25</td>
<td>0.50</td>
<td>1.00</td>
<td>3.00</td>
<td>5.00</td>
</tr>
<tr>
<td>( A_7 )</td>
<td>0.13</td>
<td>0.14</td>
<td>0.17</td>
<td>0.20</td>
<td>0.25</td>
<td>0.33</td>
<td>1.00</td>
<td>3.00</td>
</tr>
<tr>
<td>( A_8 )</td>
<td>0.11</td>
<td>0.13</td>
<td>0.14</td>
<td>0.14</td>
<td>0.17</td>
<td>0.20</td>
<td>0.33</td>
<td>1.00</td>
</tr>
<tr>
<td>( \Sigma )</td>
<td>3.65</td>
<td>4.35</td>
<td>5.34</td>
<td>8.09</td>
<td>13.92</td>
<td>22.53</td>
<td>34.33</td>
<td>46.00</td>
</tr>
</tbody>
</table>
reference values (from various sources) in trial-and-error testing, which was compared against the existing landslide inventory. The inventory was transformed into point features, because the SINMAP extension does not support polygon features input. Plausible parameters adopted as final ones (Tab. 1) were used to construct the final model. Continual SI model was further reclassified to standard intervals: 0–0.5 unstable = very high susceptibility, 0.5–1.0 potential instability = high susceptibility, 1.0–1.25 marginally stable = medium susceptibility, 1.25–1.5 stable = low susceptibility, > 1.5 unconditionally stable = very low susceptibility (Fig. 6b). For evaluation purposes, pixels with SI values <1.0 (first two classes) were aggregated together and compared against landslide instances, whereas pixels with SI values ≥1.0 were compared against non-landslide instances of the inventory (Tab. 4).

Finally, three basic LSA RF-based models were defined. The first model RF-1, was developed on training and testing datasets that contained all attribu-

<table>
<thead>
<tr>
<th>Model, class</th>
<th>Matching pixels</th>
<th>Mismatching pixels</th>
<th>TPrate</th>
<th>FPrate</th>
<th>AUC</th>
</tr>
</thead>
<tbody>
<tr>
<td>AHP model, non-landslide</td>
<td>232093</td>
<td>730</td>
<td>0.603</td>
<td>0.495</td>
<td>0.554</td>
</tr>
<tr>
<td>AHP model, landslide</td>
<td>745</td>
<td>152884</td>
<td>0.505</td>
<td>0.397</td>
<td></td>
</tr>
<tr>
<td>SI model, non-landslide</td>
<td>354355</td>
<td>1296</td>
<td>0.996</td>
<td>0.990</td>
<td>0.503</td>
</tr>
<tr>
<td>SI-model, landslide</td>
<td>116</td>
<td>12275</td>
<td>0.009</td>
<td>0.003</td>
<td></td>
</tr>
<tr>
<td>RF-1, non-landslide</td>
<td>301479</td>
<td>199</td>
<td>0.783</td>
<td>0.269</td>
<td>0.757</td>
</tr>
<tr>
<td>RF-1, landslide</td>
<td>540</td>
<td>83495</td>
<td>0.731</td>
<td>0.217</td>
<td></td>
</tr>
<tr>
<td>RF-2 attributes, non-landslide</td>
<td>270607</td>
<td>150</td>
<td>0.733</td>
<td>0.245</td>
<td>0.750</td>
</tr>
<tr>
<td>RF-2 attributes, landslide</td>
<td>589</td>
<td>114367</td>
<td>0.755</td>
<td>0.267</td>
<td></td>
</tr>
</tbody>
</table>
AHP-based modeling $A_1-8$ (Tab. 2), as well as all conditioning factors used in SI-based modeling, i.e. 15 attributes. The second RF-2 and third RF-3 models were developed on datasets that contained only attributes $A_1-8$ (eight attributes), and SI model parameters (seven attributes), respectively. Furthermore, for each of these three RF based models, the optimal combination of two RF algorithm parameters was defined using the corresponding training dataset and 10-fold cross-validation technique. The range of considered parameters was $\{100, 250, 500, 1000\}$ for the number of DT in forest, and $\{2, 3, 4\}$ for the number of randomly used factors, making a total of 12 combinations created for each training dataset, i.e. the total number of derived models was 36. These 36 models derived the landslide probability (probability of class 1), for each instance (30x30m cell). In order to further proceed with the landslide susceptibility reclassification and to validate the models, the threshold of 0.75 probability was chosen (based on several trial and error attempts, starting from 0.6, as a logical threshold for very high and high susceptibility combined, to 0.8, as the upper threshold, which is more appropriate for depicting very high susceptibility class only). After the optimal combination of parameters is found, for each of three RF-based models (RF-1, RF-2 and RF-3), the models are validated on the corresponding testing datasets. Considering the value of the defined threshold, only instances with landslide probability greater than 0.75 are declared as landslide and com-

**Figure 5.** Evaluation of final models presented on ROC curves.
Figure 6. Final susceptibility models: (a) AHP susceptibility model, (b) SI susceptibility model, (c) RF-1 all attributes susceptibility model, (d) RF-1 All attributes class 1 > 75% susceptibility model, (e) RF AHP-2 attributes susceptibility model, (f) RF-2 AHP attributes class 1 > 75% susceptibility model.
pared against landslide instances from the referent inventory, *i.e.* landslide instances from the corresponding testing datasets.

Since that the main objective of this research is the comparison of the RF, SI, and AHP approach, only the results of two best performing models are presented, *i.e.* RF all attributes susceptibility model – RF1 and RF AHP attributes susceptibility model – RF2 (Tab. 4 and Figs. 6c–6f).

### 6. Discussion and conclusion

Evaluation of all models was both qualitative (visual) and quantitative. The metric evaluation was based on the class reclassification, whereby VH and H susceptibility class were compared against landslide inventory. AHP model obviously overestimated Very High and High susceptibility class (too many VH or H areas outside registered landslides), while still failing to map most of the registered landslides into VH or H class (Fig. 6a). Model’s performance was relatively poor, with equally balanced false positive and false negative error rates (Tab. 4). It is not particularly reliable, as its AUC is less than 0.6. Visual impression suggests that engineering geological units and Distance to stream chiefly control the distribution of susceptibility classes (as suggested by the AHP matrices), while other factors do not contribute as much. Various variants of AHP model were examined, including even those which gave way to other factors, but the best results were still reached with the presented AHP model. In other words, this is the maximum that was possible to extract from the AHP approach for this particular study.

The SI model highly underestimated the VH and H landslide susceptibility classes, *i.e.* $FS<1$, as the areal coverage of this class was much smaller than the coverage of the others (Fig. 6b). In addition, VH and H classes fail to map registered landslides, as only $116/1412$ or 8% of landslide pixels matched. The LS class is too dominant, suggesting that most of the territory is stable, which is false, especially in the North periphery. False Negative rate is too high, togeth-er with the True Negative rate. Considering the latter, SI model could seem concurrent to AHP, especially since AUC was also similar (similarly low). However, underestimation of the VH and H susceptibility class definitively disqualifies it. Even though initial hypothesis that landslides triggered in May 2014 where mainly shallow (dismantling of the fragile weathering crusts of Paleozoic schists due to saturation) seemed appropriate and fitting for the initial SI assumptions, it is hereby confirmed the opposite.

The best performing RF model indicates a slight overestimation of VH landslide class, but generally the very good distribution of all VH and H classes across the area in comparison to the landslide inventory. The sampling strategy that was controlling the non-landslide training instances selection by engineering geological criteria was a great improvement, that reduced False Negatives, and
False Positives and prevented learning from ambiguous instances. RF models were superior in all aspects (which is obvious on the ROC curves, Fig. 5) particularly TP rates and AUC, over 0.75, which is considerably larger than in previous approaches. As for the comparison between different RF model variants, there was just some slight difference between RF-1 and RF-2 models.

It can be concluded that RF was successful in mapping registered landslides, despite their versatile typology and uneven distribution throughout the area, i.e. higher concentration in northern than in the other parts. It is unambiguously superior to expert-driven and deterministic approach, as it succeeds to map landslides where these two approaches fail. Its particular quality is in low FN rates, especially for landslide instances (500–600 VH and H pixels were matching the total of 1412 landslide pixels, which is 30–40%), since FN is more severe error type in LSA.

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References


Usporedba ekspertskog, determinističkog i pristupa strojnog učenja za ocjenu osjetljivosti na klizišta u općini Ljubovija, Srbija

Jelka Krušić, Miloš Marjanović, Mileva Samardžić-Petrović, Biljana Abolmasov, Katarina Andrejev i Aleksandar Miladinović

Procjena osjetljivosti na klizišta postaje vrlo produktivno istraživačko područje, pri čemu se prakticiraju različiti pristupi modeliranju kako bi se zonirale visoke i niske vjerojatnosti pojave klizišta. Međutim, ne postoji jasna suglasnost o tome koji je pristup najprikladniji. Razlog nedostatka općeg gledišta na izvedbu različitih pristupa mogao bi se djelomično objasniti osobitostima svake studije. Za procjenu učinkovitosti različitih pristupa neophodno je primjenjivati ih pod istim uvjetima za isto područje istraživanja. U ovome radu su istraživana tri različita pristupa, uključujući ekspertni, deterministički i pristup strojnog učenja, na području općine Ljubovija u zapadnoj Srbiji. Područje je poznato kao osjetljivo na klizišta i predstavlja dobru osnovu za procjenu odabranih metoda. Odlikuje ga kompleksna geologija i sklonost pojavi klizišta koja se obično nalaze u debeloj kori raspadanja paleozojskih formacija sastavljenih od škriljaca i meta-sedimenta. Pod ekstremnim uvjetima za aktivaciju, poput onog koji se odvijao u svibnju 2014., ove debele kore raspadanja se zasićuju i omogućuju raznovrsne pojave klizišta i bujica, na koje ćemo se usredotočiti u ovoj studiji. Primjena ekspertnog pristupa kroz analitičku hijerarhiju proces je dala grubu kartu procjene. Deterministički model, koji integrira beskonačne kosine i hidrološki model, dao je lošije rezultate u usporedbi sa ekspertnom metodom. Ovo se može objasniti time da su pretpostavke korištene u modelu bile previše jednostavne da generički modeliraju takav široki raspon tipologije klizišta. Konačno, pristup strojnog učenja, korištenjem algoritma Random Forest, dao je znatno bolje rezultate i pokazalo se da se može uspješno koristiti s raznovrsnom tipologijom klizišta na većoj prostornoj skali. Njegov AUC učinak je oko 0,75, što je znatno bolje od AUC vrijednosti druga dva modela koji su do 0,55, tj. na razini slučajnog nagađanja.

Ključne riječi: osjetljivost na klizišta, analitička hijerarhija, deterministički model, strojno učenje, Random Forest

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